National Exams December 2013

04-Geol-A5, Rock Mechanics

3 hours duration

NOTES:

1. If doubt exists as to the interpretation of any question, the candidate is urged to submit with the answer paper, a clear statement of any assumptions made.
2. This is a CLOSED BOOK EXAM. Candidates my use only one of two approved calculators candidates are permitted however, to bring to the examination room two sheets containing rock mechanics formulae and notes.
3. Questions have equal value. The grade for each question is given. It is suggested that the candidate proportion time based on the allocated value.
4. All questions require an answer in analytical and/or essay format. Clarity and organization of the written answer and any figures or sketches are important.
5. The examination has an overall value of 80 Marks: each question will be marked out of 20 marks as per the marking scheme provided.
6. **ANSWER ONLY 4 of the 5 questions that are provided. Only the first 4 questions that appear in the answer book will be marked**.
7. Selected equations, graphs and tables are given at the end of the exam paper. These may (or may not) be of assistance for some questions. Indicate the question number corresponding to any graphs or tables used at the back of the exam question sheets.
8. Hand in the exam booklet and the question booklet at the end of the exam.

Marking Scheme

(only 4 will be marked)

1. **20 marks total**
   - (a) 5 marks
   - (b) 5 marks
   - (c) 5 marks
   - (d) 5 marks
2. **20 marks total**
   - (a) 8 marks
   - (b) 2 marks
   - (c) 8 marks
   - (d) 2 marks
3. **20 marks total**
   - 20 marks total answer
4. **20 marks total**
   - (a) 10 marks
   - (b) 5 marks
   - (c) 5 marks
5. **20 marks total**
   - (a) 2 marks
   - (b) 3 marks
   - (c) 2 marks
   - (d) 5 marks
   - (e) 8 marks

Value

20 Marks **Question #1**

An underground garage (i.e. tunnel) is planned to be excavated in rock. This excavation is to be developed initially on the basis of diamond drill core data retrieved by remote drilling, as no site development currently has taken place. Based upon information which is provided:

5 Marks a. Determine the RQD for the core shown;

5 Marks b. Determine the RMR for the rock mass at the proposed development site;

5 Marks c. Determine the limiting excavation dimensions (maximum and minimum);

5 Marks d. Determine the unsupported stand-up times for these excavation dimensions and the range of rock reinforcement that would be necessary for the excavation (over the dimension ranges selected).

Given:

Core Recovery Data: As illustrated in **Figure Q1** in the accompanying core box sketch (total length of core recovered = 3.0 m).

Core Strength Data:

Unconfined Compressive Strength (S_c)(MPa)	Point Load Index (I_{s54})(MPa)
206.2	9.2*
221.4	10.2*
211.3	9.5*
203.3	8.8*
205.5	9.4*
	9.7
	8.9
	9.1
*I_s values and linked UCS values for calibration (i.e. first 5 pairs of data in table)	10.1
	9.3
	9.7
	9.0
	8.9
	9.9
	9.7

Joint Conditions:

- Two joint families identified:
- Join #1 strikes parallel to long axis of planned excavation, dips at 15° to the horizontal and joints repeat approximately every 1.5 m; surfaces of this family of joints are slightly rough/weathered and continuous, with

separation distance between the joint surfaces ranging between 1.0-1.5mm.

- Joint #2 strikes 45° to long axis of planned excavation, dips 20° and repeats at intervals of approximately 0.3m; surface of these joints are very rough and discontinuous with separation between surfaces being << 0.1 mm.

Stress / Water Conditions:

- Maximum ground stress components are expected to be horizontally directed, of magnitude less than 1.5 times vertical stress component, and uniformly distributed horizontally at the site of excavation; minimal water flow (<5L/min at low pressure) from the rock is anticipated.

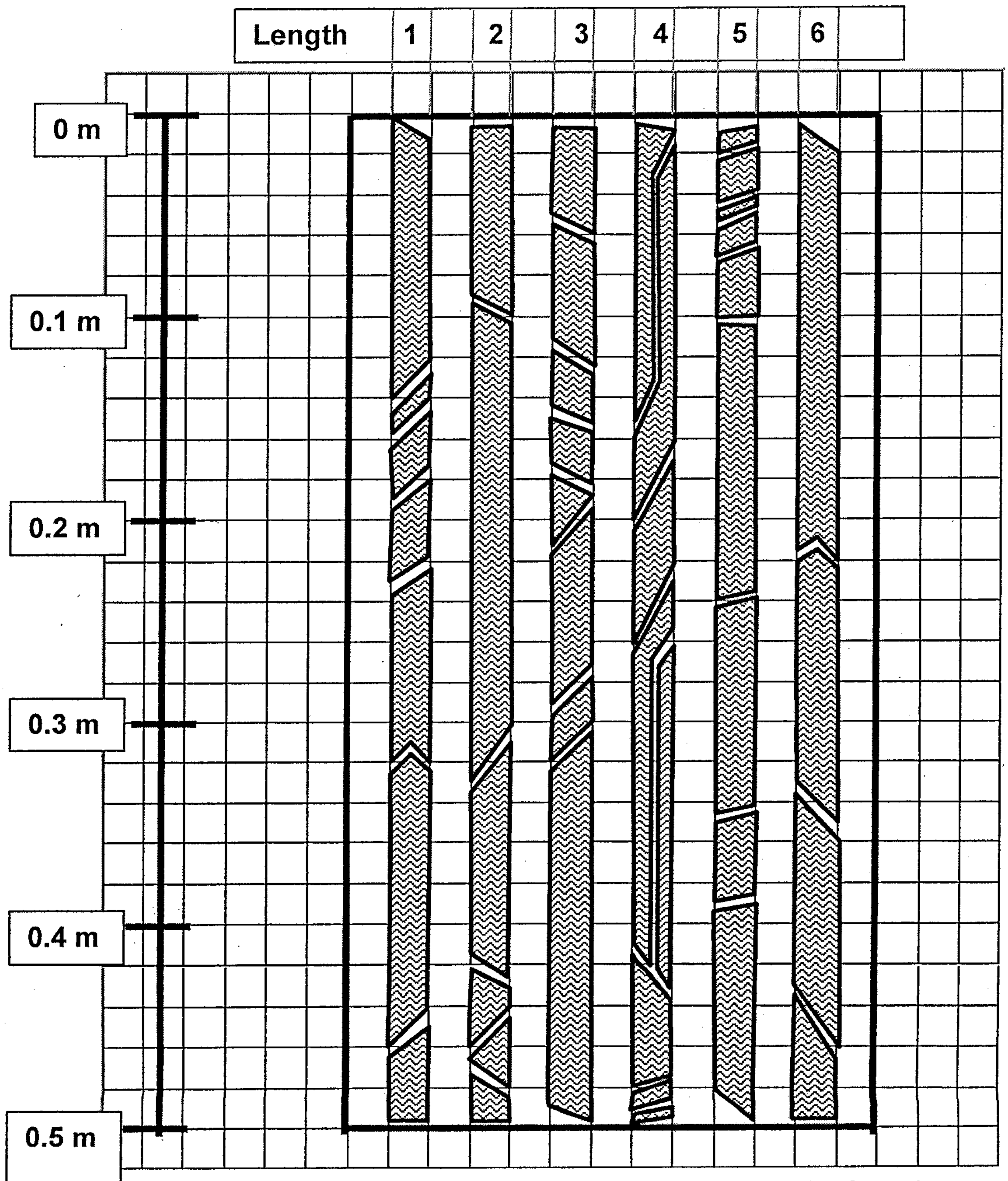


Figure Q1. Core Recovery View (3.0 metre length of core).

Value

20 Marks **Question #2**

The following triaxial compression strength test results were obtained as the result of a series of laboratory trials on core specimens recovered from a rock excavation (i.e. mine).

Confining Stress (MPa)	Failure Axial Stress (MPa)
16.8	159.3
13.2	154.5
25	198.0
9.7	140.1
20	168.0

8 Marks

a. Based on the information provided, determine the Mohr-Coulomb parameters which can be estimated to establish the limiting compression failure locus for this rock. These parameters should include strength variables as well as orientation conditions (i.e. internal angle of friction and failure angle values);

2 Marks

b. From the results of part a, what problems appear to be evident from the data that has been given?

8 Marks

c. Using the Mohr-Coulomb empirical equation relating principal stresses at failure, determine the minimum axial applied stresses that would need to be applied during triaxial failure tests to induce shear failure when confining stresses equivalent to 5, 15, 22.5 MPa are also applied;

2 Marks

d. How could one verify the accuracy of the results in part c?

Value

20 Marks **Question #3**

At depth below ground surface, geotechnical staff has noted the presence of a vertically-oriented planar fracture. It is thought that this fracture (shear) surface has developed due to the action of plane principal compressive ground stresses known to act at this depth. The major principal ground stress, under a condition of assumed biaxial loading, is known to be approximately oriented as shown in the sketch in **Figure Q3**.

On the basis of laboratory tests performed on samples of this rock material, Mohr-Coulomb strength parameters have been determined and are given as:

- (c) cohesive strength = 11.5 MPa
- (S_c) unconfined compressive strength 73.4 MPa

Using the information given, sketch a diagram which you would use to indicate the true directions of action of the major and minor biaxial stresses in a plane which is perpendicular to the strike direction of this shear feature.

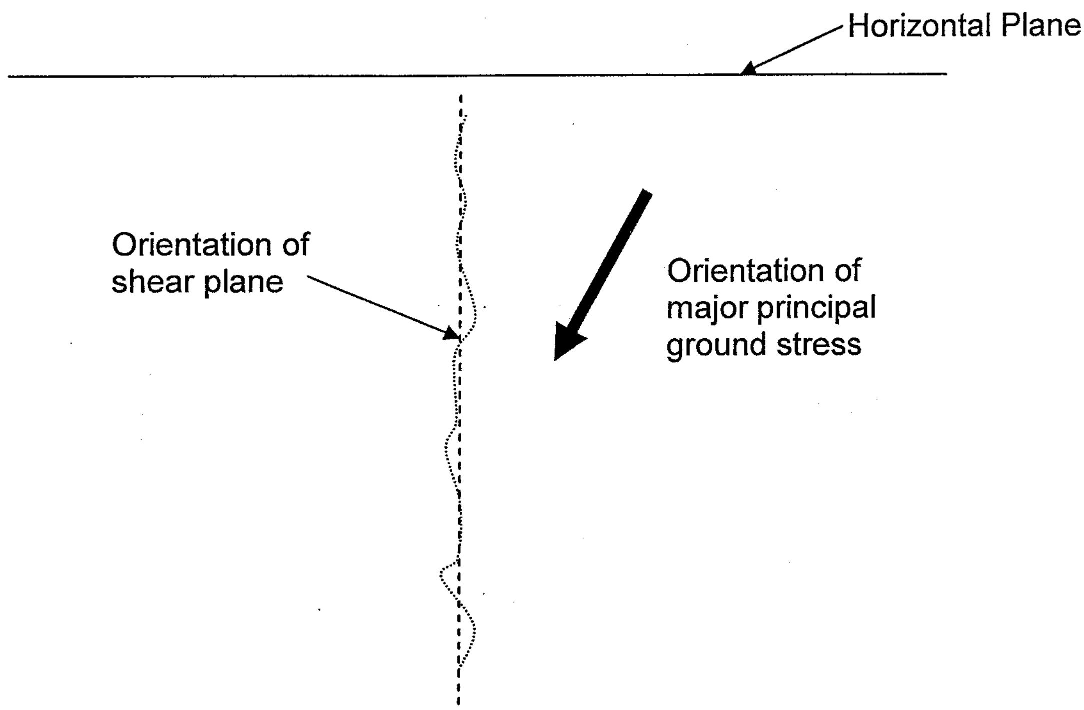


Figure Q3. Vertically-oriented planar fracture

Value

20 Marks **Question #4**

A room-and-pillar potash mine is flat-lying (i.e. horizontally bedded) and the sum of pillars which have been developed over a great horizontal width of the mine exhibit a uniform local extraction ratio (r) of 0.67. This deposit exists at a depth of 335 metres below the ground surface. The potash exhibits a Young's Modulus (E) of 13.8 GPa and a Poisson's ratio (μ) of 0.40. All pillars are square in cross-section and are assumed to be unconfined at all times.

10 Marks

a. Determine the pre-mining and post-mining vertical stress conditions on this mining horizon when it is assumed that the natural rock density in the hanging wall waste, immediately overlaying the pillar horizon, is 25.89 kN/m^3. It is also to be assumed that the horizontal pre-mining stresses are to calculated assuming full confinement conditions within the potash horizon;

5 Marks

b. If the original seam height (pillar height) was set at 5.0 metres, determine the vertical shortening of each pillar between the hanging wall contact and pillar mid-height elevations as a result of the increase in vertical stress that develops within the potash between the pre-mining and post-mining conditions. All pillars are assumed to be square in plan view and to have sides each being 7.5 metres in length;

5 Marks

c. Determine the average total transverse expansion of the pillars as a result only of the increase in the pillar vertical stress.

Value

20 Marks **Question #5**

A circular drift, 6.1 meters in diameter, is to be driven horizontally through rock in which a hydrostatic stress field exists. The uniform stress magnitude prior to development is measured to approximate 55.2 MPa. Core sample testing has indicated that the rock exhibits an unconfined compressive strength of 104.8 MPa and an internal friction angle (ϕ) of 30°.

2 Marks a. What will be the cohesive strength of this rock?

3 Marks b. Assuming that a linear Mohr-Coulomb failure locus exists, at what level of axial stress will this rock material fail if it were to be confined at a stress level of 31.2 MPa?

2 Marks c. What will be the Factor of Safety against failure for the rock present around the drift surface? Illustrate the conditions of surface stress and the safety conditions using a Mohr-Coulomb stress diagram;

5 Marks d. Determine the magnitude of internal stress that must be applied onto the drift wall surface to just induce stability at the drift wall surface;

8 Marks e. A 15 cm thick concrete lining will be placed against the drift wall surface, and a grout will be injected between the rock surface and the concrete liner. The grouting pressure that will be exerted will be equal to the calculated level of rock stress which was determined in part d. The concrete liner exhibits and unconfined compressive strength of 34.5 MPa. Sketch the pressure conditions which will act upon the concrete liner and determine the factor of safety that will exist at the liner's most critical point.

Additional Reference Material

Equations

RQD = 115 – 3.3 J_v,

Where, J_v is the sum of the number of joints per unit length for all joint (discontinuity) sets known as the volumetric joint count

$$Q = \frac{RQD}{J_n} \times \frac{J_r}{J_a} \times \frac{J_w}{SRF}$$

where RQD is the Rock Quality Designation
J_n is the joint set number
J_r is the joint roughness number
J_a is the joint alteration number
J_w is the joint water reduction factor
SRF is the stress reduction factor

Resolved Normal Stress:

$$\sigma_\theta = \frac{(\sigma_x + \sigma_y)}{2} + \frac{\{(\sigma_x - \sigma_y)(cos2\theta)\}}{2} + \tau_{xy}(sin2\theta)$$

Resolved Shear Stress:

$$\tau_\theta = \frac{\{(\sigma_y - \sigma_x)(sin2\theta)\}}{2} + \tau_{xy}(cos2\theta)$$

Point Load Test

$I_{s50} = L / D^2$

Where, L = failure compressive loading force applied (kN);
D = specimen core diameter

$S_c = 24\ (I_{s54})$ KPa

Where, S_c = unconfined compressive strength (kPa)
(I_{s54}) = index values for 5.4 cm diameter core specimens (kN/cm^2)

Mohr Coulomb Failure Criterion

$\Psi = 45^{\circ} + \varphi/2$

$S_T = C / \tan \varphi$

$(\sigma_1 + \sigma_3) / (\sigma_3 + S_T) = \tan^2 \Psi$

$\sigma_1 = \sigma_3 \tan^2 \Psi + 2C \tan \Psi = \sigma_3 \tan^2 \Psi + S_c$

Where, C = cohesion
Ψ = angle of failure plane in triaxial sample from horizontal
S_T = tensile strength
S_c = unconfined compressive strength

Mining

$\sigma_v = \text{load} / Y^2$

$\sigma_p = \text{load} / X^2$

$$\frac{\sigma_p}{\sigma_v} = \frac{A_T}{A_P}$$

Where, A_p = Post mining area
A_T = Tributary Area

$$\sigma_p = \frac{\sigma_v}{(1 - r)}$$

Where, r = extraction ratio = $(A_T - A_P) / A_T$

Kirsch Equations

$$\sigma_{rr} = \sigma/2 \{(1+k)(1-a^2/r^2) - (1-k)(1-4a^2/r^2 + 3a^4/r^4)\cos 2\theta\}$$

$$\sigma_{\theta\theta} = \sigma/2 \{(1+k)(1+a^2/r^2) + (1-k)(1 + 3a^4/r^4)\cos 2\theta\}$$

$$\sigma_{r\theta} = \sigma/2 \{(1-k)(1 + 2a^2/r^2 - 3a^4/r^4)\sin 2\theta\}$$

$$U_r = \{\mu\, r_i/E\} \bullet \{(\sigma_1 + \sigma_3) + 2(\sigma_1 - \sigma_3)\cos 2\theta$$

Where, μ = Poisson's Ratio

Thick Wall Cylinder Stress formulae

$(2P_o\text{-}P_i)=(P_i)\tan^2\Psi + S_c$

$P_i = (2P_o - S_c) / (\tan^2\Psi + 1)$

$\varepsilon_r = 1/E\,(\sigma_r - \mu\,\sigma_t) = U_r / r_i$

$U_r = \varepsilon_r\, r_i$

$U_r = \{\mu(2P_o\, r_i)\} / E$

$\sigma_t = 2(r_o^2 P_o) / (r_o^2 - r_i^2)$

Where, P_o = pre-mining hydrostatic pressure at $r = r_o$
P_i = internal pressure applied against opening surface at $r = r_i$
σ_r = radially oriented post-mining stress components, uniform for all angular directions but varying by distance away from the excavation surface.
r_i = inside radius of circular opening in rock or liner\
r_o = outside radius of installed liner or radial distance to boundary of rock media if the opening is unlined.
μ = Poisson's Ratio
U_r = inward radial displacement

Tables

Table 1. Rock Mass Rating System (After Bieniawski 1989).

A. CLASSIFICATION PARAMETERS AND THEIR RATINGS

	Parameter		Range of values				
1	Strength of intact rock material	Point-load strength index	>10 MPa	4 - 10 MPa	2 - 4 MPa	1 - 2 MPa	For this low range - uniaxial compressive test is preferred
		Uniaxial comp. strength	>250 MPa	100 - 250 MPa	50 - 100 MPa	25 - 50 MPa	5 - 25 MPa | 1 - 5 MPa | < 1 MPa
	Rating		15	12	7	4	2 | 1 | 0
2	Drill core Quality *RQD*		90% - 100%	75% - 90%	50% - 75%	25% - 50%	< 25%
	Rating		20	17	13	8	3
3	Spacing of discontinuities		> 2 m	0.6 - 2 m	200 - 600 mm	60 - 200 mm	< 60 mm
	Rating		20	15	10	8	5
4	Condition of discontinuities (See E)		Very rough surfaces Not continuous No separation Unweathered wall rock	Slightly rough surfaces Separation < 1 mm Slightly weathered walls	Slightly rough surfaces Separation < 1 mm Highly weathered walls	Slickensided surfaces or Gouge < 5 mm thick or Separation 1-5 mm Continuous	Soft gouge >5 mm thick or Separation > 5 mm Continuous
	Rating		30	25	20	10	0
5	Groundwater	Inflow per 10 m tunnel length (l/m)	None	< 10	10 - 25	25 - 125	> 125
		(Joint water press)/ (Major principal σ)	0	< 0.1	0.1, - 0.2	0.2 - 0.5	> 0.5
		General conditions	Completely dry	Damp	Wet	Dripping	Flowing
	Rating		15	10	7	4	0

B. RATING ADJUSTMENT FOR DISCONTINUITY ORIENTATIONS (See F)

Strike and dip orientations		Very favourable	Favourable	Fair	Unfavourable	Very Unfavourable
Ratings	Tunnels & mines	0	-2	-5	-10	-12
	Foundations	0	-2	-7	-15	-25
	Slopes	0	-5	-25	-50	

C. ROCK MASS CLASSES DETERMINED FROM TOTAL RATINGS

Rating	100 ← 81	80 ← 61	60 ← 41	40 ← 21	< 21
Class number	I	II	III	IV	V
Description	Very good rock	Good rock	Fair rock	Poor rock	Very poor rock

D. MEANING OF ROCK CLASSES

Class number	I	II	III	IV	V
Average stand-up time	20 yrs for 15 m span	1 year for 10 m span	1 week for 5 m span	10 hrs for 2.5 m span	30 min for 1 m span
Cohesion of rock mass (kPa)	> 400	300 - 400	200 - 300	100 - 200	< 100
Friction angle of rock mass (deg)	> 45	35 - 45	25 - 35	15 - 25	< 15

E. GUIDELINES FOR CLASSIFICATION OF DISCONTINUITY conditions

Discontinuity length (persistence)	< 1 m	1 - 3 m	3 - 10 m	10 - 20 m	> 20 m
Rating	6	4	2	1	0
Separation (aperture)	None	< 0.1 mm	0.1 - 1.0 mm	1 - 5 mm	> 5 mm
Rating	6	5	4	1	0
Roughness	Very rough	Rough	Slightly rough	Smooth	Slickensided
Rating	6	5	3	1	0
Infilling (gouge)	None	Hard filling < 5 mm	Hard filling > 5 mm	Soft filling < 5 mm	Soft filling > 5 mm
Rating	6	4	2	2	0
Weathering	Unweathered	Slightly weathered	Moderately weathered	Highly weathered	Decomposed
Ratings	6	5	3	1	0

F. EFFECT OF DISCONTINUITY STRIKE AND DIP ORIENTATION IN TUNNELLING**

Strike perpendicular to tunnel axis		Strike parallel to tunnel axis	
Drive with dip - Dip 45 - 90°	Drive with dip - Dip 20 - 45°	Dip 45 - 90°	Dip 20 - 45°
Very favourable	Favourable	Very unfavourable	Fair
Drive against dip - Dip 45-90°	Drive against dip - Dip 20-45°	Dip 0-20 - Irrespective of strike°	
Fair	Unfavourable	Fair	

* Some conditions are mutually exclusive . For example, if infilling is present, the roughness of the surface will be overshadowed by the influence of the gouge. In such cases use A.4 directly.

Table 2. Guidelines for excavation and support of 10 m span rock tunnels in accordance with the *RMR* system (After Bieniawski 1989).

Rock mass class	Excavation	Rock bolts (20 mm diameter, fully grouted)	Shotcrete	Steel sets
I - Very good rock *RMR*: 81-100	Full face, 3 m advance.	Generally no support required except spot bolting.		
II - Good rock *RMR*: 61-80	Full face, 1-1.5 m advance. Complete support 20 m from face.	Locally, bolts in crown 3 m long, spaced 2.5 m with occasional wire mesh.	50 mm in crown where required.	None.
III - Fair rock *RMR*: 41-60	Top heading and bench 1.5-3 m advance in top heading. Commence support after each blast. Complete support 10 m from face.	Systematic bolts 4 m long, spaced 1.5 - 2 m in crown and walls with wire mesh in crown.	50-100 mm in crown and 30 mm in sides.	None.
IV - Poor rock *RMR*: 21-40	Top heading and bench 1.0-1.5 m advance in top heading. Install support concurrently with excavation, 10 m from face.	Systematic bolts 4-5 m long, spaced 1-1.5 m in crown and walls with wire mesh.	100-150 mm in crown and 100 mm in sides.	Light to medium ribs spaced 1.5 m where required.
V – Very poor rock *RMR*: < 20	Multiple drifts 0.5-1.5 m advance in top heading. Install support concurrently with excavation. Shotcrete as soon as possible after blasting.	Systematic bolts 5-6 m long, spaced 1-1.5 m in crown and walls with wire mesh. Bolt invert.	150-200 mm in crown, 150 mm in sides, and 50 mm on face.	Medium to heavy ribs spaced 0.75 m with steel lagging and forepoling if required. Close invert.

Figures

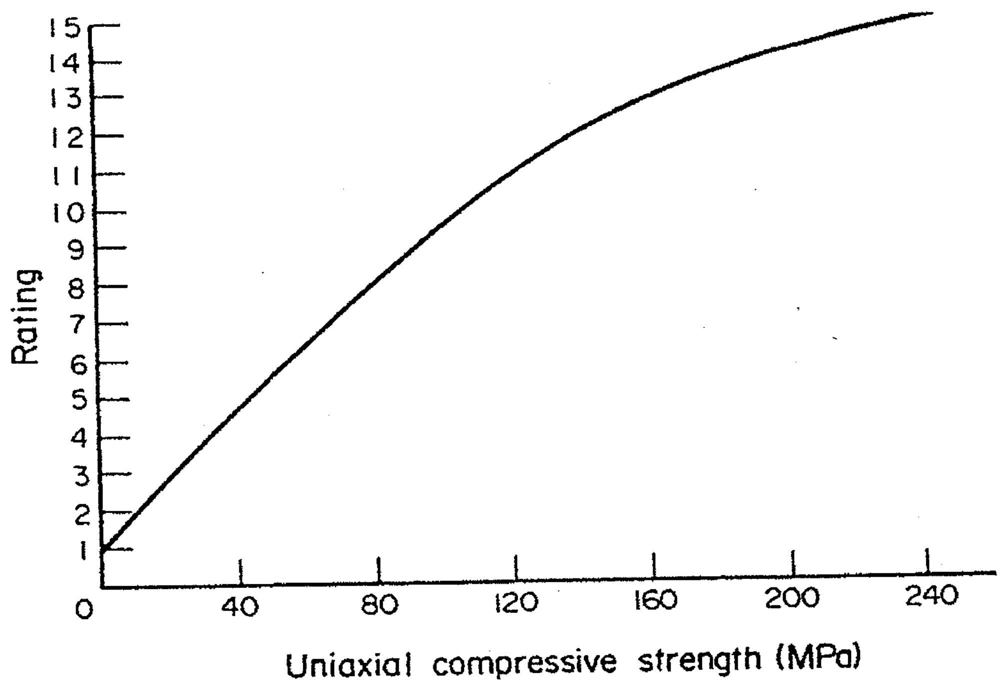


Figure 1. RMR Rating System for the strength of intact rock material

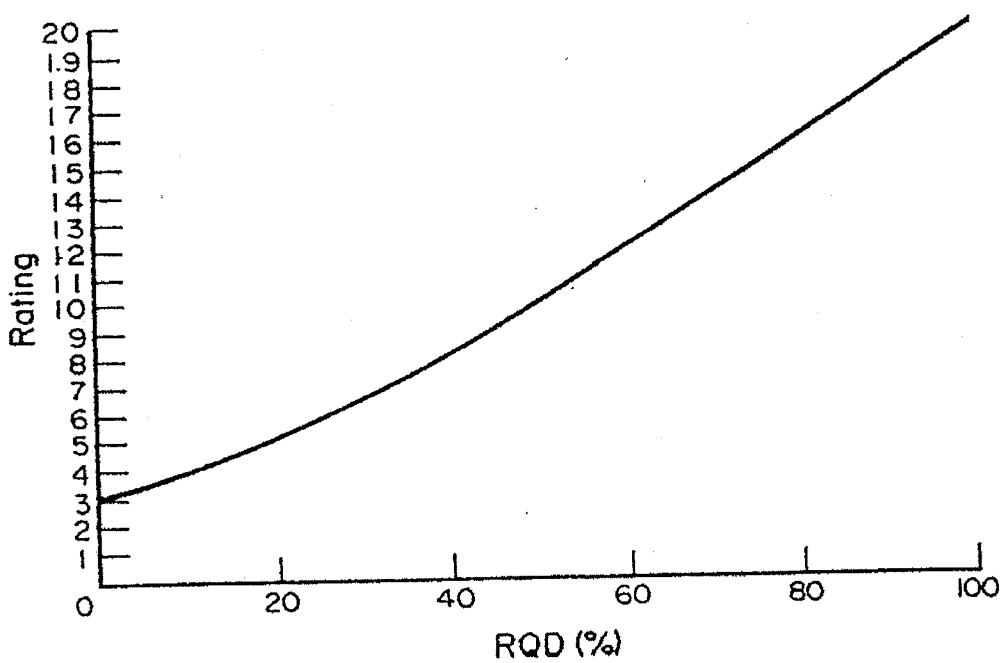


Figure 2. The RMR Rating system: ratings for RQD

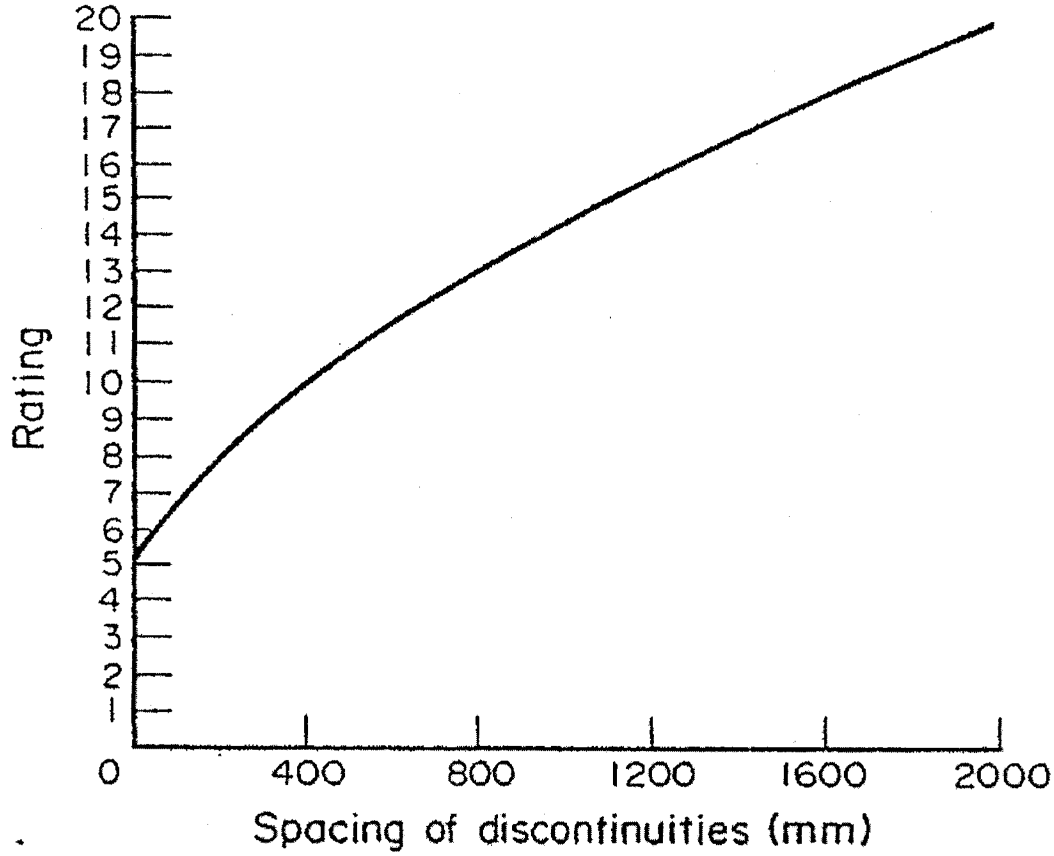


Figure 3. The RMR Rating system: ratings for Discontinuity Spacing

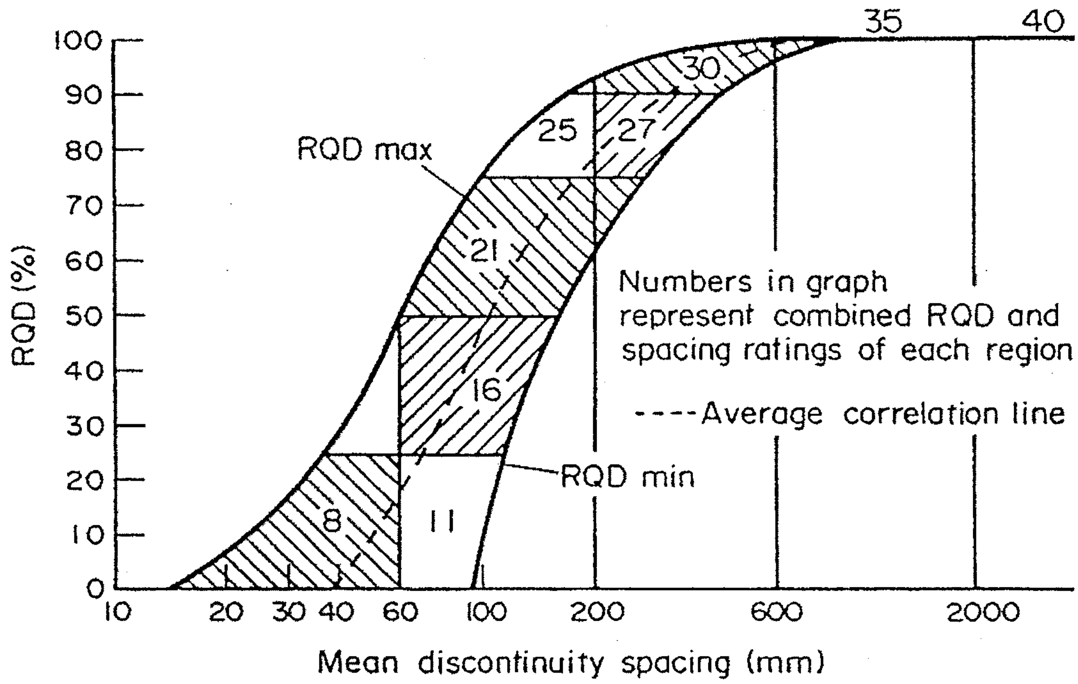


Figure 4. The RMR Rating system: Chart for correlation between RQD and Discontinuity Spacing

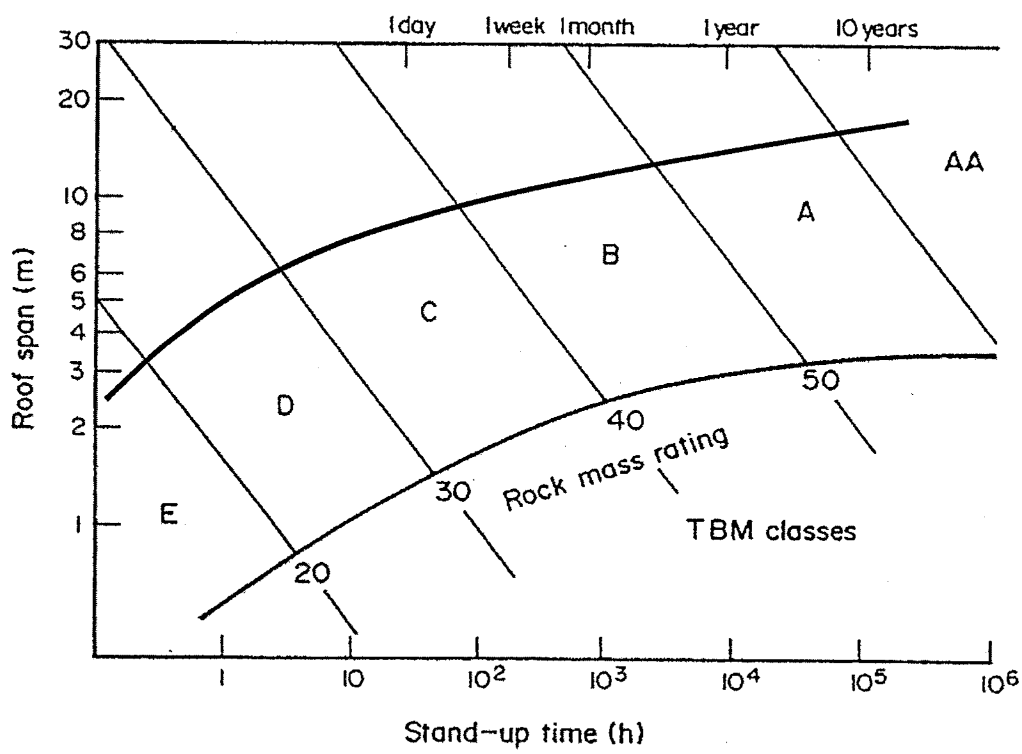
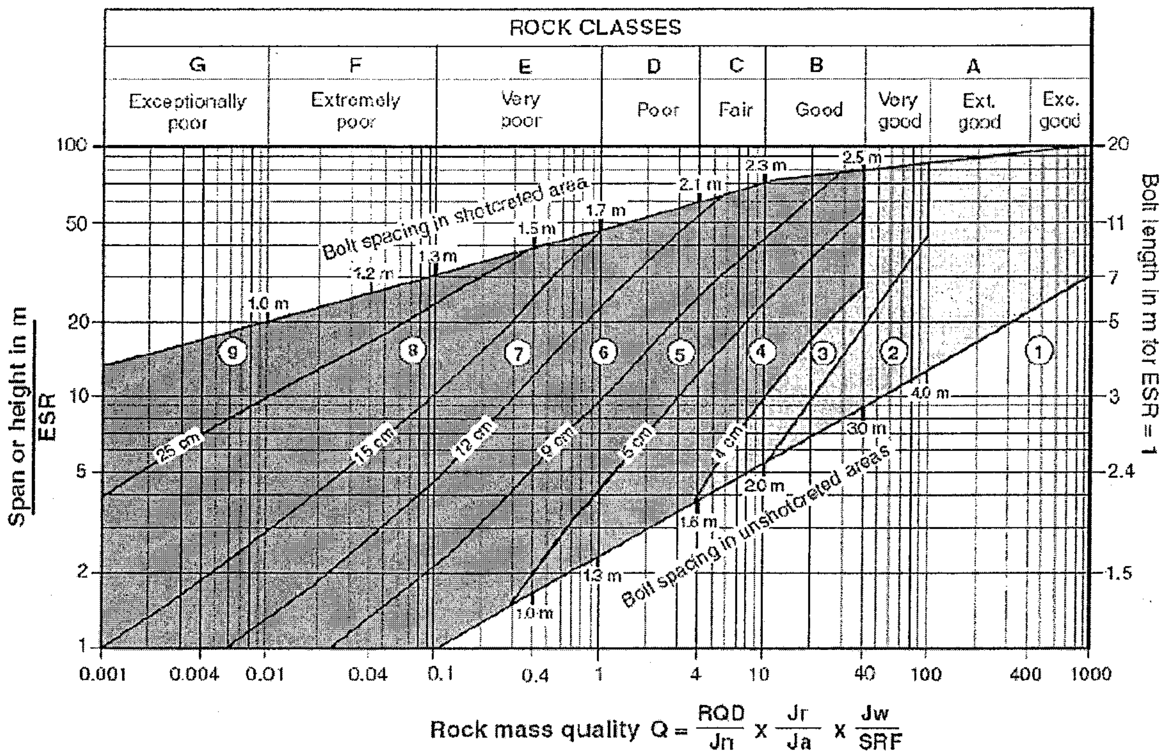


Figure 5. Modified Lauffer diagram depicting boundaries of rock mass classes for TBM applications (after Lauffer 1988).



REINFORCEMENT CATEGORIES:

1) Unsupported
2) Spot bolting
3) Systematic bolting
4) Systematic bolting, (and unreinforced shotcrete, 4 - 10 cm)
5) Fibre reinforced shotcrete and bolting, 5 - 9 cm
6) Fibre reinforced shotcrete and bolting, 9 - 12 cm
7) Fibre reinforced shotcrete and bolting, 12 - 15 cm
8) Fibre reinforced shotcrete, > 15 cm, reinforced ribs of shotcrete and bolting
9) Cast concrete lining

Figure 6. Estimated support categories based on the tunnelling quality index *Q* (After Grimstad and Barton, 1993, reproduced from Palmstrom and Broch, 2006).